

Project	2-Phase Design Spreadsheet		By
Calculation:	BS 5951-4		Approved:
Sheet Number:	1		Date:

Section Details			
Geometry	L	2000	Applied?
Grade	62.75	Depth	0
Top Flange Width B_f	60 mm	Top Scaffolding Lip D_{ts}	20 mm
Bottom Flange Width B_b	60 mm	Bottom Scaffolding Lip D_{bs}	20 mm
Effective Length L_e	2000 mm	Thickness t	7 mm
Fixed end Conditions	Fixed against Torsional Rotation & Minor Axis Flex		
Type of Intermediate Connections	Substantial		

Loading Details			
Ultimate Positive Bending Moment About Y Axis M_u	10	kNm	
Type of Bending	Single Curvature		
Applied Shear Force F_v	0	kN	
Bending Moment at Position of Max Shear $M_{u,v}$	0	kNm	
Shear Force at Position of Max Moment $F_{v,m}$	0	kN	

Results									
	A	L	L _e	B	r_{yy}	r_{zz}	A_{eff}	I_{yy}	I_{zz}
Element 1	42.0	19.0	60.0	26.0	200.0	20.0	90.0	21.0	
Element 2	124.0	191.0	60.0	42.0	200.0	60.0	97.0	120.0	
Element 3	200.0	121.0	60.0	160.0	200.0		160.0	121.0	
Element 4	160.0	107.0	60.0	160.0	200.0		160.0	107.0	
Element 5	42.0	14.0	60.0	20.0	200.0		42.0	14.0	
Total	370.0	434.0	60.0	570.0			770.0	434.0	

Effective Length to Breadth Ratio			
Utilisation	0.000		Acceptable
Moment Capacity			
Moment Capacity at Tension Face	$M_{u,t}$	0.01 kNm	
Moment Capacity at Compression Face	$M_{u,c}$	0.01 kNm	
Moment Capacity	$M_{u,max}$	0.01 kNm	
Utilisation	0.000		Acceptable
Buckling Resistance			
Resistant Effective Length Factor	k	0.00	
Slenderness Ratio	λ	40.25	
Smaller End Moment	$M_{1,y}$	0.00 kNm	
Larger End Moment	$M_{2,y}$	0.00 kNm	
Ratio of End Moments	β	0.00	
Moment Ratio Factor	$C_{m,y}$	0.75	
Form Coefficient	η	0.00	
Yield Moment	M_y	0.27 kNm	
Distortional Lateral Buckling Resistance Moment	$M_{D,y}$	0.24 kNm	
Factor α_{LT}	α_{LT}	0.25	
Buckling Resistance Moment	$M_{b,y}$	0.01 kNm	
Utilisation	0.000		Acceptable

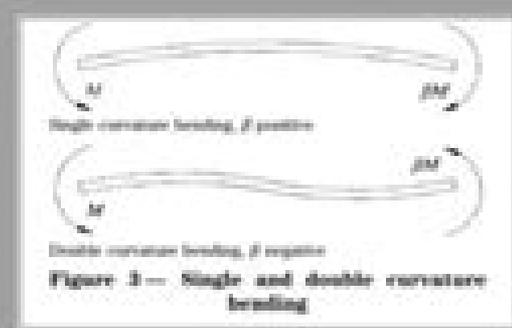
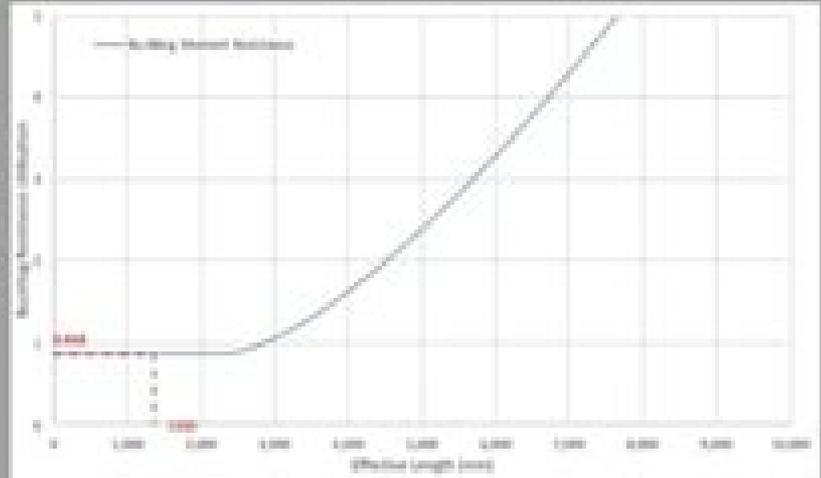
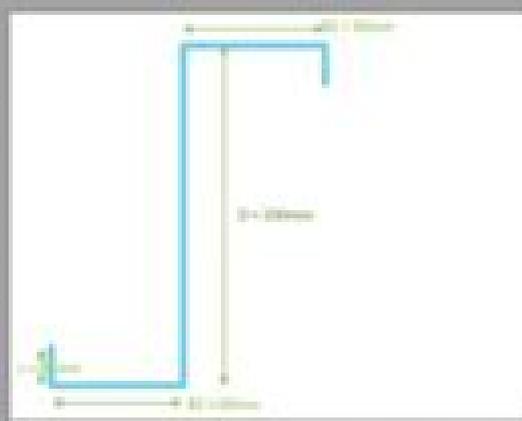


Figure 3— Single and double curvature bending

Cold Formed Steel Design By Spreadsheet Program

Prof. Sham Tickoo



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