

Project	2-Frame Design Spreadsheet		By
Calculation:	BS 5951-4		Approved:
Sheet Number:	1		Date:

Section Details

Geometry: I, 200x200, spaced?

Grade	GL	Depth	200 mm
Top Flange Width B_f	60 mm	Top Scaffolding Lip D_{st}	20 mm
Bottom Flange Width B_b	60 mm	Bottom Scaffolding Lip D_{sb}	20 mm
Effective Length L_e	2500 mm	Thickness t	7 mm

Part and Conditions: Vertical steel against Torsional Rotation & Minor Axis Flex

Type of Intermediate Connection: Substantial

Loading Details

Ultimate Positive Bending Moment About Y Axis M_y	10 kNm
Type of Bending	Single Curvature
Applied Shear Force F_v	0 kN
Bending Moment at Position of Max Shear M_{sv}	0 kNm
Shear Force at Position of Max Moment F_{sv}	0 kN

Results

	A	L	L ₁	L ₂	T ₁	T ₂	A ₁	A ₂
Element 1	42.0	19.0	6.5	26.0	250.0	20.0	90.0	20.0
Element 2	124.0	191.0	6.4	42.0	247.0	60.0	107.0	120.0
Element 3	206.0	121.0	6.9	160.0	270.0	60.0	106.0	120.0
Element 4	166.0	107.0	11.0	160.0	269.0	60.0	106.0	120.0
Element 5	42.0	14.0	6.8	20.0	252.0	62.0	14.0	
Total	374.0	434.0	60.7	572.0	1712.0	412.0		

Effective Length to Breadth Ratio			
Utilisation	0.000	Acceptable	
Moment Capacity			
Moment Capacity at Tension Face	$M_{t,y}$	10.91 kNm	
Moment Capacity at Compression Face	$M_{c,y}$	10.91 kNm	
Moment Capacity	$M_{y,red}$	10.91 kNm	
Utilisation	0.000	Acceptable	
Buckling Resistance			
Resistant Effective Length Factor	k_y	0.00	
Slenderness Ratio	λ	61.25	
Smaller End Moment	$M_{1,y}$	0.00 kNm	
Larger End Moment	$M_{2,y}$	10.00 kNm	
Ratio of End Moments	β	0.00	
Moment Ratio Factor	$C_{m,y}$	0.75	
Form Coefficient	η	0.00	
Yield Moment	M_y	12.27 kNm	
Disturber Later of Buckling Resistance Moment	M_d	60.24 kNm	
Factor α	α	16.25	
Buckling Resistance Moment	$M_{b,y}$	10.91 kNm	
Utilisation	0.000	Acceptable	

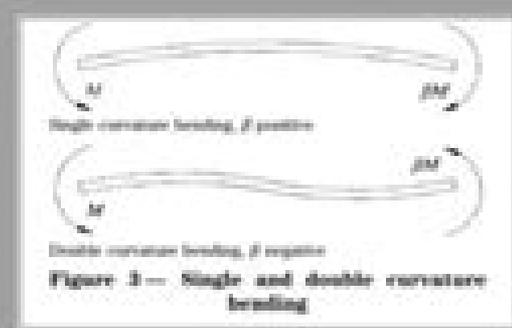
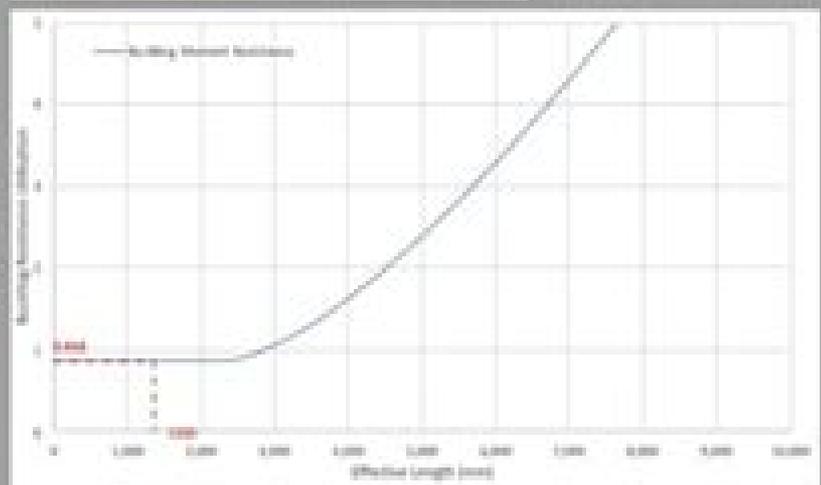
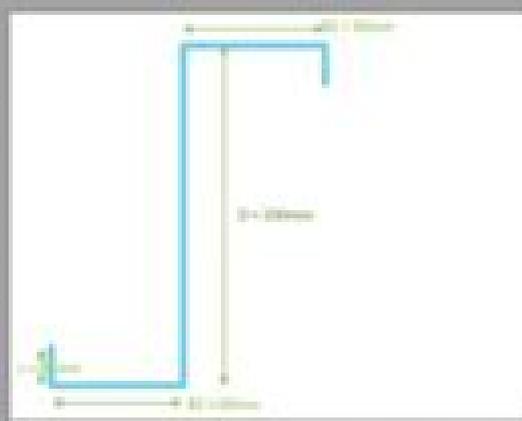


Figure 3— Single and double curvature bending

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Lei Huang



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