

Principal Stress & Maximal Shear Stress

2-Dimensional

$$\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\tau_{max} = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\sigma_{x'} = \frac{\sigma_x + \sigma_y}{2} + \left(\frac{\sigma_x - \sigma_y}{2}\right) \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\sigma_{y'} = \frac{\sigma_x + \sigma_y}{2} - \left(\frac{\sigma_x - \sigma_y}{2}\right) \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\tau_{x'y'} = -\left(\frac{\sigma_x - \sigma_y}{2}\right) \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$\theta = \frac{1}{2} \tan^{-1} \left(\frac{2\tau_{xy}}{\sigma_x - \sigma_y} \right)$$

$$I_{xx} = \int y^2 dA, I_{yy} = \int x^2 dA, I_{xy} = \int xy dA, I_{xx'} = I_{xx} \cos^2 2\theta - I_{xy} \sin 2\theta + I_{yy} \sin^2 2\theta$$

3-Dimensional

Determinants Equations (Stress Invariants)

$$I_1 = \sigma_x + \sigma_y + \sigma_z$$

$$I_2 = \sigma_x \sigma_y + \sigma_x \sigma_z + \sigma_y \sigma_z$$

$$I_3 = \sigma_x \sigma_y \sigma_z - \tau_{xy} \tau_{yz} - \tau_{yz} \tau_{zx} - \tau_{zx} \tau_{xy}$$

$$I_4 = \sigma_x \tau_{xy} \tau_{yz} + \sigma_y \tau_{yz} \tau_{zx} + \sigma_z \tau_{zx} \tau_{xy} - \tau_{xy}^2 \sigma_z - \tau_{yz}^2 \sigma_x - \tau_{zx}^2 \sigma_y$$

$\sigma_{1,2,3}$ = roots of $\lambda^3 - I_1 \lambda^2 + I_2 \lambda - I_3 = 0$

$$\tau_{1,2,3} = \sqrt{\frac{I_1^2 - 3I_2}{3}}$$

See ex. for principal direction

$$\tau_{1,2,3} = \sqrt{\frac{I_1^2 - 3I_2}{3}}$$

$$\tau_{1,2,3} = \sqrt{\frac{I_1^2 - 3I_2}{3}}$$

Centroidal Bending

$$\sigma_{max} = \frac{M}{I} c$$

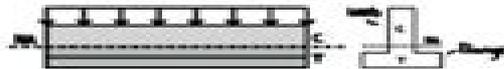
c is distance from NB to point of interest; it differs for concave and convex

I is NB with the MCT neutral axis

bending stress will be MCT (symptical) axis

M is axial (symptical) stress distribution

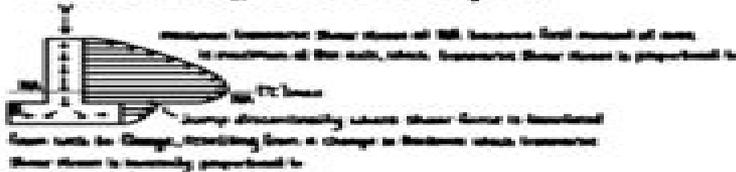
tensile bending \rightarrow positive, compression bending \rightarrow negative



Transverse Shear

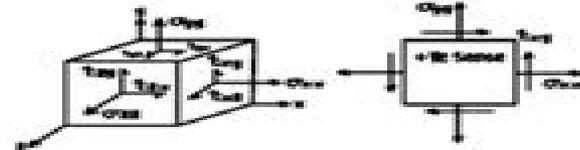
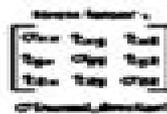
$$\tau = \frac{VQ}{It}$$

Linear \rightarrow Linear, for rectangular along axis but I & Q are with respect to I & Q will be MCT axis opposite to V (another axis (symptical))



Torsion

$$\tau = \frac{T\rho}{J}$$



Case Sectional Properties

$$Centroid: \bar{y} = \frac{\sum y_i A_i}{\sum A_i}$$

Second Moment of Area

$$I = \sum I_{xxi} + \sum A_i d_i^2$$

$$d_i = (y_i - \bar{y})$$

If I_{yy} , same but in face perspective of y -axis

If I_{xy} , \dots \dots

Area Moment of Inertia

$$J = \sum (I_{xxi} + I_{yyi})$$

$I_x = \sum (I_{xxi} + A_i d_i^2)$ if bar is used as a beam

First Moment of Area

moment of area above or below point of interest

$$Q = A \bar{y}$$

where A is the remaining case sectional area above or below that point

and \bar{y} is the distance between the centroid of that area to the centroid of the entire case section

2-Dimensional Plane Stress & Strain

Stress

(1) — one dimension compared to other two dimensions is very small

like a plate or a shell, stress with large constant

(2) — total has to be in the plane of the plate, etc., same way it does not

Strain

(1) — one dimension compared to other two dimensions is very large

(2) — symmetric loading condition

Stress-Strain

$$\begin{bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} \\ C_{21} & C_{22} & C_{23} \\ C_{31} & C_{32} & C_{33} \end{bmatrix} \begin{bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{21} & E_{22} & E_{23} \\ E_{31} & E_{32} & E_{33} \end{bmatrix} \rightarrow \begin{bmatrix} E_{11} & \nu_{12} & \nu_{13} \\ \nu_{21} & E_{22} & \nu_{23} \\ \nu_{31} & \nu_{32} & E_{33} \end{bmatrix}$$

Stress-Strain Relationship

$$E_{11} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33}$$

$$E_{22} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33}$$

$$E_{33} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33}$$

$$E_{12} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33}$$

$$C = G(1 + \nu)$$

Plane Stress Condition

$$E_{11} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33}$$

$$E_{22} = E \nu_{12} \nu_{13} + \nu_{12} \nu_{13} E_{33}$$

$$E_{33} = -\nu_{12} \nu_{13} E_{33} - \nu_{12} \nu_{13} E_{33}$$

$$E_{12} = E \nu_{12} \nu_{13}$$

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Robert R. Redfield



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Course Notes Structural Mechanics Mechanical Book Review: Unveiling the Magic of Language

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