

Project	2-Phase Design Spreadsheet		By
Calculation:	BS 5951-4		Approved:
Sheet Number:	1		Date:

Section Details			
Geometry	L	2000	Support
Grade	62.75	Depth	0
Top Flange Width B_f	60 mm	Top Scaffolding Lip D_{t1}	20 mm
Bottom Flange Width B_b	60 mm	Bottom Scaffolding Lip D_{b1}	20 mm
Effective Length L_e	2000 mm	Thickness t	7 mm
Fasten and Conditions	Fasten and Against Torsional Rotation & Minor Axis Flex		
Type of Intermediate Connections	Substantial		

Loading Details			
Ultimate Positive Bending Moment About Y Axis M_u	10	kNm	
Type of Bending	Single Curvature		
Applied Shear Force F_{sd}	0	kN	
Bending Moment at Position of Max Shear M_{u1}	0	kNm	
Shear Force at Position of Max Moment F_{sd1}	0	kN	

Results									
	A	L	L ₁	B	T_{eff}	L_{eff}	A_{eff}	L_{eff}	
Element 1	42.0	19.0	66.7	26.0	250.0	20.0	90.0	20.0	
Element 2	124.0	97.1	66.8	42.0	267.0	65.0	97.1	120.0	
Element 3	266.0	121.5	67.0	66.0	276.0	106.0	121.5		
Element 4	366.0	97.5	67.0	66.0	285.0	166.0	97.5		
Element 5	42.0	34.3	67.0	26.0	292.0	42.0	34.3		
Total	1240.0	434.4	67.7	278.0	1723.0	433.0			

Effective Length to Breadth Ratio			
Utilisation	0.000		Acceptable
Moment Capacity			
Moment Capacity at Tension Face	$M_{u,y}$	9.91 kNm	
Moment Capacity at Compression Face	$M_{u,c}$	9.91 kNm	
Moment Capacity	$M_{u,max}$	9.91 kNm	
Utilisation	0.000		Acceptable
Buckling Resistance			
Resistant Effective Length Factor	k_y	0.90	
Slenderness Ratio	λ	61.25	
Smaller End Moment	$M_{1,y}$	0.00 kNm	
Larger End Moment	$M_{2,y}$	9.90 kNm	
Ratio of End Moments	β	0.00	
Moment Ratio Factor	$C_{m,y}$	0.75	
Form Coefficient	η	0.90	
Yield Moment	M_y	12.27 kNm	
Distur. Later. of Buckling Resistance Moment	$M_{0,y}$	60.24 kNm	
Factor α_{LT}	α_{LT}	0.25	
Buckling Resistance Moment	$M_{b,y}$	9.91 kNm	
Utilisation	0.000		Acceptable

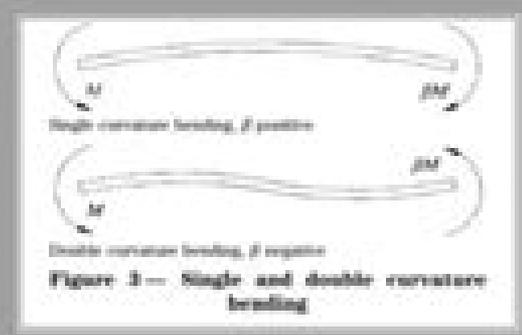
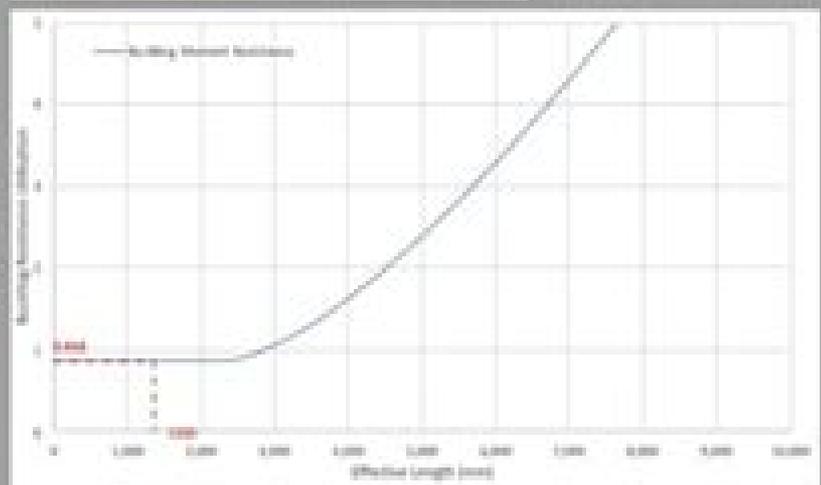
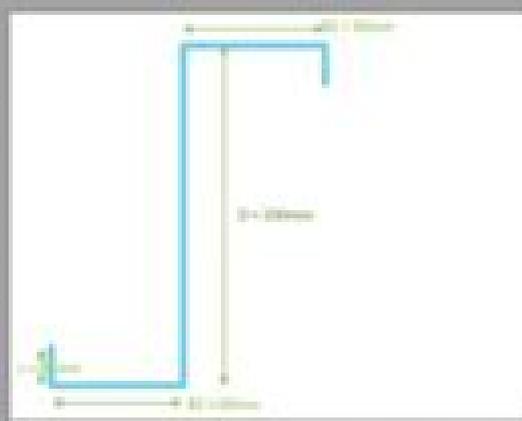


Figure 3— Single and double curvature bending

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Scott C. Dulebohn



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